

where,

$$x_{ij} = x_i - x_j$$

$$y_{ij} = y_i - y_j$$

and t is the thickness of the element, E is the Young's modulus, A is the area of the triangle and, ν is the Poisson's ratio. Young's modulus determines the flexibility of the object, whereas the Poisson's ratio defines the relation between lateral strain and strain along the direction of loading (Hence, smaller values of Poisson's ratio indicates that the object will not stretch much under tension).

Similarly, the bending stiffness matrix ($[k^e_b]$) can be expressed as the multiplication of two matrices (Rao, 1989);

$$[k^e_b] = [N^{-1}]^T [A] [N^{-1}]$$

where,

$$[N] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 1 & 0 & y_2 & 0 & 0 & y_2^2 & 0 & 0 & y_2^3 \\ 0 & 0 & 1 & 0 & 0 & 2y_2 & 0 & 0 & 3y_2^2 \\ 0 & -1 & 0 & 0 & -y_2 & 0 & 0 & -y_2^2 & 0 \\ 1 & x_3 & y_3 & x_3^2 & x_3 y_3 & y_3^2 & x_3^3 & (x_3^2 y_3 + x_3 y_3^2) & y_3^3 \\ 0 & 0 & 1 & 0 & x_3 & 2y_3 & 0 & (2x_3 y_3 + x_3^2) & 3y_3^2 \\ 0 & -1 & 0 & -2x_3 & -y_3 & 0 & -3x_3^2 & -(y_3^2 + 2x_3 y_3) & 0 \end{bmatrix}$$

and A is a symmetric matrix ($A[i][j] = A[j][i]$, $i = 1, \dots, 9$; $j = 1, \dots, 9$) with elements described as

$$A[1][1] = 0$$

$$A[2][1] = A[2][2] = 0$$

$$A[3][1] = A[3][2] = A[3][3] = 0$$

$$A[4][1] = A[4][2] = A[4][3] = 0$$

$$A[4][4] = 2x_3 y_2$$

$$A[5][1] = A[5][2] = A[5][3] = A[5][4] = 0$$

$$A[5][5] = x_3 y_2 (1 - \nu)$$

$$A[6][1] = A[6][2] = A[6][3] = A[6][5] = 0$$

$$A[6][4] = 2\nu x_3 y_2$$

$$A[6][6] = 2x_3 y_2$$

$$A[7][1] = A[7][2] = A[7][3] = A[7][5] = 0$$

$$A[7][4] = 2x_3^2 y_2$$

$$A[7][6] = 2\nu x_3^2 y_2$$

$$A[7][7] = 3x_3^3 y_2$$

$$A[8][1] = A[8][2] = A[8][3] = 0$$

$$A[8][4] = \frac{2}{3} x_3 y_2 (\nu x_3 + (y_2 + y_3))$$

$$A[8][5] = \frac{2}{3} (1 - \nu) x_3 y_2 (x_3 y_2 + (y_2 + y_3))$$

$$A[8][6] = \frac{2}{3} x_3 y_2 (x_3 y_2 + \nu (y_2 + y_3))$$

$$A[8][7] = \frac{1}{2}x_3^2 y_2 \{2vx_3 y_2 + (y_2 + 2y_3)\}$$

$$A[8][8] = \frac{1}{3}x_3 y_2 \{(3-2v)(x_3^2 y_2) + (2-v)(x_3 y_2)(y_2 + 2y_3) + (3-2v)(y_2^2 + y_2 y_3 + y_3^2)\}$$

$$A[9][1] = A[9][2] = A[9][3] = A[9][5] = 0$$

$$A[9][4] = 2vx_3 y_2 (y_2 + y_3)$$

$$A[9][6] = 2x_3 y_2 (y_2 + y_3)$$

$$A[9][7] = \frac{3}{2}(vx_3^2 y_2)(y_2 + 2y_3)$$

$$A[9][8] = \frac{1}{2}(x_3^2 y_2)(y_2 + 2y_3) + vx_3 y_2 (y_2^2 + y_2 y_3 + y_3^2)$$

$$A[9][9] = 3x_3 y_2 (y_2^2 + y_2 y_3 + y_3^2)$$

2. Computer Implementation of Assembly of Overall Stiffness Matrix (Adapted from Rao, 1989)

If K is stored as a symmetric square matrix

```

initialize K (set the elements of the K matrix to zero)
for i = 1: number of triangles
    for j = 1:3
        ij = the index of the j-th node of the i-th triangle
        for k = 1:3
            ik = the index of the k-th node of the i-th triangle
            K[ij][ik] = K[ij][ik] + Ki [j][k]
        end
    end
end

```

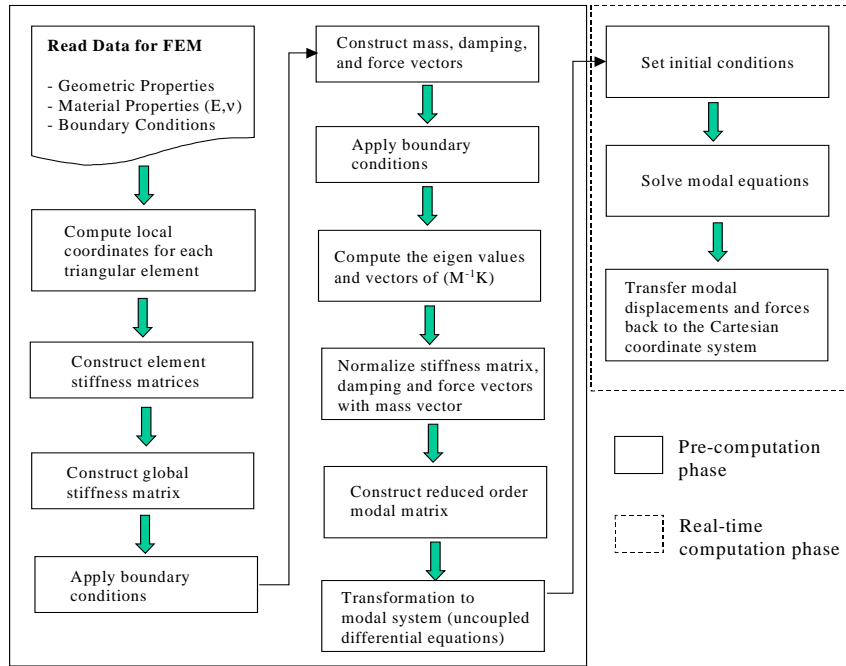
K can also be stored as a banded matrix to save from the memory space. If K is going to be stored as a banded matrix, we need to know the band-width of the matrix to construct it. The band-width is equal to the maximum difference between the numbered degrees of freedom of the structure plus one.

```

initialize K (set the elements of the K matrix to zero)
for i = 1: number of triangles
    for j = 1:3
        ij = the index of the j-th node of the i-th triangle
        for k = 1:3
            ik = the index of the k-th node of the i-th triangle
            ikm = ik - ij + 1
            if (ikm < 1)
                continue;
            else
                K[ij][ikm] = K[ij][ikm] + Ki [j][k]
            end
        end
    end
end

```

3. Flow chart of the finite element computations



APPENDIX B: Numerical Integration Using Newmark Procedure

Note that

- the damping matrix assumed to be linearly proportional with mass matrix in the original equations, $B = \alpha M = \text{diag}(\alpha m_i)$
- the steps for “Direct Numerical Integration of Original System” (DNIOS) and the “Numerical Integration of Reduced Modal System” (NIRMS) are slightly different from each other. These differences were clearly marked in the algorithm below.

1. Pre-computation phase:

a) Construct stiffness and mass matrices: K and $M = \text{diag}(m_i)$

b) Define the initial conditions:

$$U_0, \dot{U}_0, \ddot{U}_0$$

(DNIOS)

$$X_0 = \Phi U_0, \dot{X} = \Phi \dot{U}_0, \ddot{X} = \Phi \ddot{U}_0$$

(NIRMS)

c) Calculate the constants of “Newmark” integration

$$a_0 = \frac{4 + 2\alpha \Delta t}{\Delta t^2}$$

$$a_1 = a_0$$

$$a_2 = \frac{4}{\Delta t} + \alpha$$

$$a_3 = 1$$

$$a_4 = \frac{4}{\Delta t^2}$$

$$a_5 = -a_4$$

$$a_6 = a_5(\Delta t)$$

$$a_7 = -1$$

$$a_8 = \frac{\Delta t}{2}$$

$$a_9 = \frac{\Delta t^2}{4}$$

$$a_{10} = a_9$$

d) Construct the modified stiffness matrix:

$$\hat{K} = K + a_0 M \quad (\text{DNIOS})$$

$$\hat{K} = K + a_0 I = \text{diag}(\omega_i^2 + a_0) \quad (\text{NIRMS})$$

e) Decompose the modified stiffness matrix using Cholesky decomposition (see Numerical Recipes, Press et al.) (Only for DNIOS)

f) Compute the modal force vector (Only for NIRMS)

$${}^{t+\Delta t} f^R = (\Phi^R)^T {}^{t+\Delta t} F$$

2. Real-time simulation phase (Computational steps for direct integration of equilibrium equations)

Step 1: Construct modified force vector

$$\hat{F} = {}^{t+\Delta t} F + M(a_1 {}^t U + a_2 {}^t \dot{U} + a_3 {}^t \ddot{U}) \quad (\text{DNIOS})$$

$$\hat{f}^R = {}^{t+\Delta t} f^R + a_1 {}^t X^R + a_2 {}^t \dot{X}^R + a_3 {}^t \ddot{X}^R \quad (\text{NIRMS})$$

Step 2: Compute modified displacements using

$$\hat{K} \hat{U} = \hat{F} \quad \text{“Cholesky backsubstitution” (see Press et al, 1993)} \quad (\text{DNIOS})$$

$$\hat{X}^R = \hat{f}^R / \hat{K} \quad (\text{NIRMS})$$

Step 3: Compute accelerations, velocities, and displacements at time $t + \Delta t$

$$\begin{aligned} {}^{t+\Delta t} \ddot{U} &= a_4 \hat{U} + a_5 {}^t U + a_6 {}^t \dot{U} + a_7 {}^t \ddot{U} \\ {}^{t+\Delta t} \dot{U} &= {}^t \dot{U} + a_8 ({}^t \ddot{U} + {}^{t+\Delta t} \ddot{U}) \end{aligned} \quad (\text{DNIOS})$$

$${}^{t+\Delta t} U = {}^t U + \Delta t {}^t \dot{U} + a_9 {}^t \ddot{U} + a_{10} {}^{t+\Delta t} \ddot{U}$$

$$\begin{aligned} {}^{t+\Delta t} X^R &= a_4 \hat{X}^R + a_5 {}^t X^R + a_6 {}^t \dot{X}^R + a_7 {}^t \ddot{X}^R \\ {}^{t+\Delta t} \dot{X}^R &= {}^t \dot{X}^R + a_8 ({}^t \ddot{X}^R + {}^{t+\Delta t} \ddot{X}^R) \end{aligned} \quad (\text{NIRMS})$$

$${}^{t+\Delta t} \ddot{X}^R = {}^t \ddot{X}^R + \Delta t {}^t \dot{\ddot{X}}^R + a_9 {}^t \ddot{X}^R + a_{10} {}^{t+\Delta t} \ddot{X}^R$$

$${}^{t+\Delta t} U = (\Phi^R) {}^{t+\Delta t} X^R$$

Step 4: Compute the forces that will be reflected to the user

$${}^{t+\Delta t} F = M {}^{t+\Delta t} \ddot{U} + B {}^{t+\Delta t} \dot{U} + K {}^{t+\Delta t} U \quad (\text{DNIOS})$$

$$\begin{cases} {}^{t+\Delta t} f^R = {}^{t+\Delta t} \ddot{X}^R + \alpha {}^{t+\Delta t} \dot{X}^R + \Omega^2 {}^{t+\Delta t} X^R \\ {}^{t+\Delta t} F = (\Phi^R) {}^{t+\Delta t} f^R \end{cases} \quad \text{where, } \Omega^2 = \text{diag}(\omega_i^2) \quad (\text{NIRMS})$$

Step 5: Goto Step 1.